# STETTLER REPORT

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# STETTLER REPORT, 1959

# INTRODUCTION

A study of the groundwater resources of the Stettler area was undertaken by the Research Council of Alberta in 1957, after the town of Stettler requested assistance to determine whether large quantities of groundwater could be obtained in the vicinity of Stettler. This report deals with the groundwater geology and hydrology in the vicinity of Stettler; it will be included in a more comprehensive report of the entire Stettler area to be published later this year. This report is not intended for public distribution although the information and conclusions contained herein are available to anyone upon request.

### GROUNDWATER GEOLOGY AND HYDROLOGY

# Geology and Groundwater

Groundwater geology is the study of those physical properties of rocks which control the occurrence of groundwater. All rocks, either unconsolidated or consolidated, which lies below the water table, below a depth of about 10 feet at Stettler, are saturated with water which occupies the small interstices or open spaces between the particles making up the rock. Where these interstices are interconnected, the water is free to move through the rock under the influence of differential hydrostatic pressure and the rock is said to be permeable. Because the permeability of the material that a well is completed in directly determines the rate at which water may be produced, geologic studies are directed mainly toward locating highly permeable strata by studying the physical characteristics of the rocks themselves and their depositional and post-depositional history. Regional changes in permeability are readily delinected by geologic methods, these methods however, are less adaptable to determine local changes in permeability which may also be very significant.

Stattler is underlain by consolidated sedimentary rocks (commonly termed "bedrack")
Which geologists have named the Edmonton formation. These sediments were deposited

around the margin of the sea which covered Alberta several million years ago. The Edmonton formation is overlain by unconsolidated sediments deposited by continental glaciers which advanced over Alberta in the past and melted or retreated from this area about 10,000 years ago. During the time interval following the retreat of the sea which covered Alberta and preceding the advent of glaciation the bedrock surface was eroded by streams and rivers forming a mature topographic surface characterized by low upland areas separating broad valleys. The Buffalo Lake channel which trends eastward through Buffalo Lake is one of these ancient channels, which has been completely filled with glacial deposits so that its course is no langer evident from the surface topography.

The more permeable sediments, for example, sand and gravel deposits in the glacial drift and sandstone beds in the Edmonton formation through which water is readily transmitted are termed aquifers. Geologic mapping has been carried out employing subsurface data to delineate the major potential aquifers in the Stettler area.

The Edmonton formation is the uppermost bedrock formation in the Stettler area. It is made up of interbedded sandstone, sandy shale and shale, with minor coal seams and Ironstone concretions. Formations underlying the Edmonton formation contain water that is brackish to salty and unfit for human consumption. The upper surface of the bedrock is an erosional surface, as already described, which slopes downward to the north toward the Buffalo Lake channel. The Buffalo Lake channel is the major bedrock channel; tributary channels include the Erskine channel and the Leahurst channel.

The Edmonton formation dips westward at about 20 feet per mile. Consequently the depth to a given marker horizon in the Edmonton formation will increase to the west.

The Edmonton formation has been subdivided into three members, the upper Edmonton member, the middle Edmonton member, and the lower Edmonton member. The upper Edmonton

member has been eroded from all but the extreme southwest part of the Stettler area. The middle Edmonton member is the most productive part of the Edmonton formation, and where it forms the subcrop, wells yielding up to 60 gpm. may be completed in the bedrack. The Edmonton formation dips west, and the erosional surface of the bedrack slopes downward toward the north. Therefore, the middle Edmonton member has been removed by erosion east of a northwest trending line which passes about one mile east of Stettler. East of this line water wells completed in the bedrack obtain water from the lower Edmonton member, and generally wells may be expected to yield less than 5 gpm.

The top of the lower Edmonton member at Stettler is a rock unit about 20 feet thick consisting of dark grey to black, moderately hard shale which may contain numerous fassils, and up to four beds of black limestone about one foot thick. It may be recognized in the cutting samples from test holes and by the change in drilling rate, as it is penetrated by the drill for it is generally harder than the overlying and underlying material. This rock unit constitutes an important marker horizon at Stettler, as it is very unlikely that sufficient water for a municipal supply well will be obtained at a greater depth from the lower Edmonton member. The only bedrock aquifer in which municipal supply wells can be economically located is the middle Edmonton member.

The Edmonton formation is overlain by glacial drift deposited over the irregular bedrock surface. The thickness of glacial drift is extremely variable, ranging from 5 to 50 feet thick south of Stettler, and from 100 to 260 feet thick north of Stettler. The glacial drift is made up principally of clayey till, containing minor lenses of sand and gravel. The only potential aquifers in the glacial drift are:

- (a) sand and gravel deposits along the course of bedrock channels, now buried by up to 250 feet of glacial drift,
- (a) outwash sand deposits, deposited during the last phase of deglaciation, overlying glacial drift.

Occurrences of both types were tested during this study. A total of 12 test holes were drilled to test the sand and gravel deposits in the Buffalo Lake channel, 10 miles north of Stettler, and at the east end of Buffalo Lake.

About 20 feet of very fine grained sand was encountered immediately overlying the bedrock in five test holes drilled at the east end of Buffalo Lake. The sand deposit found at the east end of Buffalo Lake is discontinuous as no sand was found in two test hales. The other five test holes drilled to test the channel directly north of Stettler did not encounter any sand or gravel in the channel. At the sites tested, it is unlikely that wells capable of yielding more than a few gallons per minute could be developed. Further test drilling to locate coarser granular deposits along this channel is not recommended at this time. An extensive outwark sand deposit in Tp. 38, Rs. 20 and 21, was also tested by drilling at seven sites.

This outwark sand deposit was found to be made up principally of fine-grained sand, in which high capacity wells could not be completed.

The only bedrock channels in the area which were not test drilled are the Erskine channel which extends northward from Erskine to Buffaio Lake, and the Leahurst channel which is now occupied by Red Willow Creek. Both channels are minor tributaries to the Buffalo Lake channel.

There appears to be little well sorted coarse sand or gravel associated with the glazial deposits in the Stettler area, and therefore, there is little chance of obtaining large quantities of groundwater from the glacial drift. The only possibilities not yet investigated include the tributory bedrock channels, and the possibility of obtaining induced infiltration from Buffalo lake through shallow and deposits around the southern share of the lake. Either of these prospects would require extensive testing to evaluate their potential.

# Hydrology

The regional hydrology of the Edmonton formation is relatively straightforward as the entire formation behaves as a single homogeneous aquifer. The plezometric surface map, constructed by contouring the elevation to which water rises in wells throughout the area, shows that the configuration of the plezometric surface conforms to the present day topography and that the regional direction of groundwater movement is northward toward the Buffala Lake channel. This bedrock channel behaves as an underground drain carrying groundwater eastward to the Battle River. Red Willow Creek is an effluent stream throughout its course; that is, groundwater discharges into the creek rather than water from the creek seeping into the ground. The plezometric surface has a definite gradient and the groundwater flow system is in dynamic equilibrium, that is, on a regional scale the recharge to the flow system is balanced by a corresponding discharge from the flow system. Because the plezometric surface conforms to the present topography this recharge must be due to local infiltration of water to the groundwater system on upland areas, and discharge from the groundwater flow system in lowland areas.

The amount of recharge is not known but it is probably low (in the order of one inches per year). There are no water level records available from the Stettler area to indicate whether or not any regional changes in the elevation of the piezometric surface have occurred because of climatic variations in the average annual precipitation. It is felt, however, that regional variations due to this cause will be very small.

At Stettler groundwater withdrawal since 1905 has disrupted the dynamic equilibrium of the groundwater flow system and created a pronounced depression in the plezometric surface. In the vicinity of Stettler, all groundwater movement is directed toward the centers of pumping within the town. The maximum velocity of groundwater movement in the Edmonton formation in the cone of influence is very low, estimated to be less than 0.5 feet per day, except in the immediate vicinity of producing wells where it would be somewhat higher.

All the town supply wells obtain water from the middle Edmonton member. The wells are artesian, that is the water rises above the depth at which it is encountered, and characteristically have a low available drawdown, that is the water does not rise for above the top of the aquifer.

The parameters employed to quantitatively evaluate the productivity of an aquifer are the transmissibility and the storage coefficient. The transmissibility, T, is the product of the permeability and the thickness of the aquifer; in its numerical form it is expressed as the number of gallons which may be transmitted through a vertical strip of aquifer one foot wide in one day, under a hydraulic gradient of 1 foot per foot. The storage coefficient is a dimensionless expression of the volume of water released from storage, from a vertical prism of unit cross-sectional area having a height equal to the thickness of the aquifer, when the hydrostatic pressure is reduced one foot. These aquifer coefficients may be determined directly from a pumping test from the drawslown observed in one or more observation wells at a known radius of the pumping well. In this study the transmissibility and storage coefficient were determined from pumping test data by the Theiss non-equilibrium equation; the mathematical expression for this is contained in Appendix B.

# Summary

Field investigations to date have shown that there is no aquifer in the Stettler area in which wells capable of yielding more than 100 gpm. could be completed. Further, it is unlikely that additional exploration would materially alter this conclusion. It is therefore certain that the middle Edmonton member aquifer represents the best prospect for the development of additional groundwater for the town of Stettler for the following reasons:

- (a) additional development may take place near the town,
- (b) wells yielding up to 60 gpm. may be located by proper testing procedures,

- (c) exploration and well completion costs are low,
- (d) this aquifer may be expected to produce water of the same quality as that currently utilized.

Before proceeding with a detailed discussion of the geology and hydrology of the middle Edmonton member aquifer at Stettler, a brief review of the groundwater exploration since 1957 is in order.

# GROUNDWATER EXPLORATION, 1957

Well No. 11 (Fig. 1), located in the southwest part of Stettler, was drilled in July, 1957 and placed on production in January, 1958, after extensive testing had been completed. The sample log and completion history are included in Appendix A. The well was initially drilled to a depth of 300 ft. to test the lower Edmonton member which was found to be unproductive. Consequently, the hole was plugged back to 150° and the well completed in the middle Edmonton member.

Three pumping tests were run, at 26 g.p.m., for three days, 40 g.p.m. for one day, and 60 g.p.m. for 10 hours. The average aquifer coefficients obtained were used to determine that the maximum safe pumping rate should not exceed 39 g.p.m. The pump was adjusted to this rate in 1958, however, because the well cannot be produced continuously at this time as the available reservoir storage is limited, the average continuous production is now about 26 g.p.m.

The well was properly completed and developed; the pumping tests indicate a moderately high well efficiency. The well efficiency does not appear to have decreased significantly in two years of operation. The drilling and testing of this well completed the exploration activity for 1957.

An automatic water level recorder was installed on Well No. 6 to provide a continuous record of water level fluctuations in the Stattler well field. Meters were installed on all supply wells in 1956 and daily production records have been maintained since that time.

# GROUNDWATER EXPLORATION, 1958

The writer outlined a test drilling program to consist of three test holes (Fig. 1) to evaluate the possibility of locating additional supply wells on the east side of town. This testing program was planned to complete the evaluation of the amount of groundwater available within the town limits that could be developed without new extensions to the distribution system.

The testing program conclusively indicated that:

- (a) supply wells could not be completed east of Main St. because of changes in the geology,
- (b) little advantage is to be gained by drilling replacement wells at the same location as old supply wells in which the production has markedly declined.

STETTLER 1958-1: This test hole, located in the northeast part of town was drilled, tested, and abandoned in August 1958. The hole produced less than 1 g.p.m. on a bail test. The lower Edmonton member is unproductive at this location also. Sand was encountered in the glacial drift in this test hole but was not tested; the sand is fine-grained, and probably has a limited areal distribution.

STETTLER 1958-2: (Well #1-A) This test hole was drilled 50 feet northeast of Well No. 1 which is the original supply well, to see whether production could be increased from new wells drilled adjacent to old wells. This well was completed at a depth of 162 feet in the middle and lower Edmonton members, and is capable of producing about 3 g.p.m. This

production rate is not significantly greater than that of Well No. 1 prior to its abandonment, although Well No. 1 was reported to be originally capable of producing 15 g.p.m. It is concluded that the decline in production of Well No. 1 is due principally to local depletion of the aquifer rather than to deterioration of the well.

STETTLER 1958-3: (Well #12) This test hole, located adjacent to the new skating rink, was drilled to a depth of 140 feet. This well produces entirely from the middle Edmonton member, as the top of the lower Edmonton member was ancountered at 120 feet. A pump test was conducted and from this the maximum safe continuous production was calculated to be 36 g.p.m. The pump was adjusted to this setting and the well placed on production on October 6, 1953.

After Well No. 12 was placed an production, Well No. 5 was abandoned. The production history of No. 5 well had been unsatisfactory because of the law well efficiency accused excessive drawdown and cavitation. Chemical and bacteriological analyses of water from this well in 1957 showed a measurable nitrate concentration, but no bacteriological contamination. Because no other wells at Stettler yield water containing nitrates, it was felt that the nitrate concentration may have been caused by direct contamination of the well from an adjacent surface drain because the well was not properly completed. Consequently, when the well was abandoned it was grouted from the bottom up with a cement—sand slurry.

# GROUNDWATER EXPLORATION, 1959

Groundwater exploration in 1957 and 1958 showed that no further wells could be located within the town limits. Because any extension of the distribution system outside the town limits would entail large capital expenditures an extensive testing program was undertaken to determine the best area for developing additional groundwater resources outside the town.

A total of seven test holes were drilled and tested (Fig. 1). One test hole (1959-2) was completed as a well to provide water for the cemetery. The remaining test holes were capped and will be used as observation wells to study the long term behaviour of the aquifer.

The exploration program to date has disclosed the presence of apparently systematic variations in the transmissibility of the middle Edmonton member. The isogram map (Fig. 1), shows the transmissibility variations the transmissibility determined from pumping tests of the wells shown are indicated in brackets. Within the stippled areas wells may be completed which have an above average productive capacity. The high transmissibility within these zones appears to be due mainly to a greater number of sandstone beds present, and to a lesser extent due to the higher permeability of individual sandstone beds encountered.

Two zones of higher transmissibility have been discovered. Both are narrow (about 1/2 mile wide) and trend northwest. Both zones appear to be continuous along trend, but some variation in transmissibility may be expected to occur along trend.

The most easterly zone passes through the west side of Stettler, Wells No. 8, 9, 10, 11, and 12 are completed in this zone. South of Stettler, the cemetery well is also completed in this zone. Further development along the trend of this high permeability zone is not recommended for the following reasons:

- (1) the thickness of the middle Edmonton member decreases rapidly northwest of Stettler, where the upper part has been removed by erosion,
- (2) southeast of Stattler is somewhat more favorable but it is anticipated that the maximum production per well would not be greater than 30 g.p.m. because there is insufficient available drawdown, the most productive part of the equifor being very near the surface,

(3) additional supply wells completed in this zone within or near Stettler would interfere with existing supply wells; the total production would not be significantly increased.

Another zone of higher transmissibility passes through the SW.1/4, Sec. 1, Tp. 39, R. 20, W. 4 Mer. Three test holes were completed in this zone, 1959-5, 1959-6 and 1959-7; 1959-1 was drilled just east of the higher permeability development.

Wells in this area have a higher available drawdown because the most productive part of the middle Edmonton member is encountered at a greater depth, and the static water level is nearer the surface than in the town of Stettler. This means that wells in this area can be safely produced at a higher rate than those in Stettler. This is the best area for future groundwater development located during this testing program. Production testing indicates that a well field can be developed in the SW. 1/4, Sec. 1, Tp. 39, R. 20.

It should be pointed out that the Intervening zones of lower permeability do not behave as hydrologic boundaries as there is still groundwater movement through the low permeability zones. The productive capacity of any well in this aquifer depends primarily upon the transmissibility of the aquifer in the immediate vicinity of the well and is not greatly influenced by changes in transmissibility at greater distances from the well. The sole purpose of locating wells only in zones of higher transmissibility is to increase the operational efficiency of the well field by permitting withdrawal of the same amount of water from the fewer wells producing at a higher rate.

### EVALUATION OF HYDROLOGIC DATA

The aquifer coefficients were determined from several pumping tests. The best pumping tests results were obtained from a three-day test conducted on test hole 1759-5, using Test italians 1959-7, 1959-6, 1959-1 and two private wells as observation wells. The aquifer coefficients calculated from this test are: transmissibility T = 3120 gpcf, and storage coefficient,

 $S_r = 9.0 \times 10^{-4}$ . Because this test was conducted under better conditions than any other pumping test, it is felt that these values for the equifer coefficients obtained provide the most reliable values for the equifer coefficients for the higher permeability zones. These values are closely comparable to the values obtained from pump tests of wells 11 and 12, using No. 6 as an observation well which yielded calculated values of T = 4000 gpdf and  $S = 4.6 \times 10^{-4}$ . The values for the equifer coefficients T = 3120 gpdf and  $S = 9.0 \times 10^{-4}$  are employed for all further calculations.

The equifer coefficients determined may be employed to predict the short term performance of wells in this equifer, however, it is felt that the predicted well performance is likely to be too pessimistic when production is considered over a period of many years. The major source of error occurs in the determination of the storage coefficient.

The storage coefficient may also be calculated if the valume of the same of depression caused by production, and the total production from the well field can be determined. At Stettler there is insufficient data and only a crude approximation of the cone of depression can be obtained. Likewise, the cumulative water production from wells within the town can only be crudely approximated, since production records have been maintained only since 1956. The storage coefficient calculated ranges from  $1.0 \times 10^{-1}$  to  $5 \times 10^{-2}$ . This calculated value is larger by a factor of 100 than the value for the storage coefficient determined from pumping tests.

The influence of production from town supply wells on the observation wells was calculated in order to obtain what would be in effect a longer term pumping test. The drawdown component of Viells 7, 8, 9 and 10, Well No. 5, Well No. 11, Well No. 12, and the trailer court well were calculated separately and combined to produce the calculated drawdown curve of Well No. 5 shown in Figure 4. The observed dopth to water curve was compiled from water level records maintained on this well.

The calculated curve follows the observed curve closely until June 1959, thereafter the observed curve falls above the calculated curve. More data are required before it can be determined whether this departure is significant, however, this type of departure would be expected to occur if there is recharge to the equifer by leakage of the confining backs of the equifer.

If there is water entering the aquifer in this way the cone of influence of the well field will have a finite radius and production wells will drawdown and approach an equilibrium at which the water produced is balanced by vertical leakage. It may require several years to approach equilibrium if the production remains constant.

The amount of vertical leakage is very small. If it is assumed that production prior to 1946 was obtained entirely from storage without vertical leakage, then the amount of vertical leakage since 1945 would be in the order of 0.002 gallons/day ft<sup>2</sup>. This is a very low figure, and from the geological characteristics of the confining bads it is entirely conceivable that this amount of leakage can and does occur.

It is concluded therefore, that the transmissibility of the middle Edmonton member, in the zones of higher transmissibility discovered to date is about 3120 gpcif. The storage coefficient of the aquifer is about  $9.0 \times 10^{-4}$ , but it is almost certain that the aquifer is not an ideal aquifer but that there is vertical leakage, which in effect is directly added to the storage coefficient.

Hydrologic calculations based upon these equifer coefficients provide a prediction of the future aquifer performance that represents a conservative estimate.

### DESIGN OF PROPOSED WEST-STETTLER WELL FIELD

# Design

The most efficient well field will be one from which the maximum amount of water may be withdrawn from the aquifer by means of the minimum number of wells. In order to

achieve this design it is necessary to completely understand the hydrologic behavior of the aquifer when wells are pumped for long periods of time. At Stattler there is insufficient data upon which to base an exact design, however, if it is assumed that the aquifer is an ideal artesion aquifer, and that all water produced is derived from storage within the aquifer that is there is no recharge or leakage to the aquifer, then the design will be safe as this is the least favorable combination of factors that may occur.

The aquifer coefficients determined from the pumping test of Test Hole 1959-5,

(T = 3120 gpdf and S = 9.0 x 10<sup>-4</sup>) were used to design the well field. The individual pumping rate, total daily well production and well spacing, for various combinations of wells producing at different rates are shown in Table 1. It is evident that the optimum combination is to have two production wells, producing 60 lgpm (173,000 lgpd), spaced 2300 feet apart. The recommended location for these wells is on an east-west line immediately south of the Highway No. 12 right-of-way, in Lsd. 5 and 6, Sec. 1, Tp. 39, R. 20, W. 4th Mer., at the locations shown on Figure 2. This alignment will locate the wells perpendicular to the direction of groundwater flow.

If production wells are completed at the locations shown and produced continuously at 60 gpm each, then a cone of influence will develop rapidly around the well field. Figures 2 and 3 show the progressive development of the cone of influence, after production has continued for about 100 days, and 5 years. It is immediately evident from these figures that the cone of influence predicted from the assumption that the equifer is an ideal artesian equifer, is much deeper and more extensive than the cone of influence areated by production within the existing Stettler well field. For this reason it is fall that the calculated drawdown or interference represents the maximum possible interference, and that the actual amount of interference will be much smaller or non-existant.

The actual configuration of the cone of influence should be observed as pumping progresses by means of periodic water level measurements in the existing observation wells. This will serve two purposess

- (a) the maximum actual interference at any point within the cone of influence may be estimated,
- (b) the amount of vertical leakage or recharge may be determined.

If the leakage is significant it may be feasible to locate two additional production wells pumping at about the same rate in the SW. 1/4, Sec. 1, to increase the total daily production to about 300,000 gpd. This, however, cannot be determined until production has continued for two to three years.

Table I
Production and Well Spacing Calculations

Number of Wells	individual pumping rate	V/el  spacing	Total daily production	Length of gathering system	Length of gathering system (feet) 1,000 gals. production
1	80	0	115,000	11,000 *	95.5
2	<b>60</b>	2,300	173,000	13,300	77.0
3	60	>10,000	260,000	>31,000	>119
3	50	10,000	216,000	31,000	143
4	40	6,000	230,000	29,000	126

<sup>\*</sup> Approximate distance from the existing water tower to the NE. corner Ltd. 6, Sec. 1, Tp. 39, R. 20, W. 4 Mer.

. 16 -

## RECOMMENDED DEVELOPMENT PROCEDURE

It is recommended that two production wells be completed at the locations shown on Figure 2. These wells should be completed with 3-5/8 inch O.D. steel surface casing cemented or driven to bedrock, and finished either open hole or with a slotted liner, to a total depth of 160 to 170 feet. Each well, after an initial pumping test, should be equipped with a pump capable of producing 60 to 80 lpgm. Each well should be permanently equipped with a flow meter and an air-line gauge.

The maximum amount of hydrologic data will be obtained by the following procedure:

- (1) canduct one to/week production tests on each well separately, using the other production wells and existing test holes as observation wells,
- (2) after the wells are connected to the distribution system, the pumping rate of the wells should be adjusted so that these wells will produce sufficient water to supply the entire system when they are pumped continuously,
- (3) all wells in the town can then be shut down except for emergency production, unless one well is required for pressure maintenance in the distribution system in the north-west part of town.
- (4) this distribution of production should be maintained for as long as possible, at least for several months,
- (5) weekly water level measurements should be made in all observation wells, and production wells.

This procedure will supply the following data:

- (a) the actual configuration of the cone of influence in the west Stattler wall field as pumping progresses will be measured, from this the facility of locating additional production wells in the SW. 1/4, Sec. 1 may be studied,
- (b) a recovery test of the entire Stettler well field will be possible, this should

indicate how much water has been removed from storage, and provide additional data on the extent of the cone of influence, and whether permanent changes due to aquifer compaction have occurred.

# LONG TERM GROUNDWATER DEVELOPMENT

A continuing program of groundwater exploration should be established to evaluate groundwater prospects at a greater distance from Stettler. This program should be set up in much the same way that the 1959 testing program was carried out, by drilling one or two test hales each year.

The following points should be considered:

- (a) a competent water well driller should be employed for all testing operations, drilling should be carried out either by the cable-tool method, or rotary method,
- (b) the Edmonton formation is very variable, so test holes should be located on approximately 1/2 mile spacing,
- (c) 4-1/2" or 5" surface casing should be set to bedrock either driven or comented to prevent surface water contamination,
- (d) test holes should be drilled to the top of the lower Edmonton member, which should be found at a depth of 150 to 250 feet depending upon the location and ground elevation; a formation log should be kept of each hole.
- (e) after drilling, each well should be pump tested, at a rate of up to 40 gpm for several hours, drawdown and recovery measurements should be made on the pumping well.
- (f) all test holes drilled should be capped and maintained as observation wells and water level measurements taken periodically; if at all possible the elevation of the top of the surface casing should be determined by direct levelling.

If such a program is carried out, it will be possible to outline creas in which the proposed well field can be extended to obtain additional groundwater as the demand increases.

The information gained will permit the most efficient development of the available groundwater resources in the area.

APPENDIX A

WELL LOGS

TEST HOLE No.: Stattler 1957-1

LCCATION: Corner of 46 Ave. and 57 St.

WELL: No. 11

3

CROUND ELEVATION: 2679

DATE COMPLETED: July 28/57

DRILLER: Roy Forrester, Red Deer

EQUIPMENT: Cable-tool rig

SURFACE CASING: 8 5/8" O.D. Steel insert joint, 36.5" casing driven to 35".

WELL COMPLETION: Slotted liner 0'-160', slotted 8" x 1/4" 2 slots/foot 45'-160'.

STATIC WATER LEVEL: 20.5°

ELEVATION: 2658.5

AVAILABLE DRAWDOWN: 17°

177-196

136-218

MAXIMUM DEFTH TO WATER: -37°

ESTIMATED MAXIMUM PUMPING RATS: 39 g.p.m.

No water coming into hole.

# LITHOLOGIC LCG

0-29	Olscial drift, silt, clay and till with a few lenses of sand.
29 <b>-36</b>	Shale, silty to sandy-grey, soft bentonitic.
36-42	Sandstone, very light gray, soft bentonitic.
1:2-4:9	Shale, light brown-grey, interbedded with sandstone, light grey.
49-61	Sendstone, light grey, very fine-grained, soft to moderately hard, inter-
10	bedded with shale, sandy, light grey, soft, and shale light brown-grey.
61-85	Shale, sandy, light grey bentonitic, interbedded with sandstone, light
	grey, very fine-grained, poorly sorted, soft, bentonitic.
85 <b>-126</b>	Shale, light grey, soft bentonitic. Dark grey shale 9 98, 110, 118.
126-132	Sandstone, light grey, very fine-grained, moderately hard.
132-153	Shale, grey, soft, shale, light brown, soft.
153-171	Shale, grey and light grey, in part silty.
171-17?	Sandstone, silty, light grey, very fine-grained, containing ironstone
	concretions.

Shale, light grey and light brown-grey, in part silty, soft and bentonitic.

Sandstone, silty, light grey, interbedded with shale, light grey. Tested.

- 218-238 Shale, light grey and light brown-grey, soft bentonitic, thin partings of coal at 224, 230, 236.
- 238-245 Sandstone, silty light grey, moderately hard, interbedded with shale, grey and greenish grey.
- 245-265 Shale, light grey, light brown-grey, coal 258 to 260.
- 265-289 Sandstone, silty, light grey, soft to moderately hard, numerous ironstone concretions, tight.
- 289-301 Shale, grey, soft with thin partings of coal.

  Total depth 301.

Hole below 6" casing totally dry. Plugged back hole to 160°. Pulled 6" casing. Ran slotted liner to 160°. Perforated two 8" x 1/4" slots/foot from 45° to 160°.

TEST HOLE No.: Stattler 1959-3

LOCATION: Just north of Irvins, W side LSD 12, Sec. 31, Tp. 38, R. 19,

WELL: Coservation 1959-3

GROUND ELEVATION: 2700

DATE COMPLETED: September 17/59

DRILLER: Hi-Rate Drilling, Calgary

F. Gertsen

EQUIPMENT: Mayhew 1000 rotary rig

SURFACE CASING: 5 1/2" O.D. steel x 27', cemented, surface return.

WELL COMPLETION: Open hole.

STATIC WATER LEVEL: -40'

AVAILABLE DRANDOWN: 13' = maximum depth to water -53'.

ESTIMATED HAXDAM BAFE PUMPING RAIN: 13 g.p.m.

# LITHOLOGIC LOG

C-13 Clay

Sand and gravel 13-17

Shale 17 -53

Sandstone 53*-*57

Shale and minor sandstone 57-139

139-160 Shale

Detail sample-log on file at R.C.A. office.

TEST HOLE No.: Stattler 1959-6

LOCATION: 188'S, 15'E, NOC. LSD. 1 Sec. 2. Tp. 39, R. 20, W. 4 Mer.

Will No.: Observation 1959-6

CROUND ELEVATION: 2670

DATE COMPLETED: October 15/59

DRILLER: Hi-Rate Drilling, Calgary

F. Certama

EQUIPMENT: Naybew 1000 Rotary rig

SURFACE CASING: 5" O.D. Steel to 43', cemented, surface return.

WELL COMPLETION: Open hole

STATIC WATER LEVEL: -3.5

AVAILABLE DRANDOWN: + 62.5 = maximum depth to water 66\*

ESTIMATED HAXIMUM SAFE PUMPING RATE: 300 g.p.m.

MOTE: A longer pumping test would probably show a maximum safe pumping rate of 100 g.p.m.

# LITHOLOGIC LOG

0-8 Clay Sard 3-17 17-19 Gravel 19-20 Clay 20-34 Sand 34-66 Shale 66-69 Sandstone

69-190

Sandstone with siltstone interbedded with shale.

Detail sample-log on file at R.C.A. office.

APPENDIX B

QUANTITATIVE METHODS

# THEISS EQUATION

The drawdown at any point in the vicinity of a discharging or recharging well in an ideal artesian aquifer may be expressed as

$$s = \frac{Q}{4 \% T} \int_{U}^{\infty} \frac{e^{-U}}{v} dv$$
 (1)

where 
$$u = \frac{r^2 S}{4 T t}$$

This may be re-written as:

$$s = \frac{114.6 \, Q}{1} \int_{0}^{\infty} \frac{e^{-U}}{U} du$$
 (2)

$$u = 1.56 \frac{r^2 S}{T t}$$

s = drawdown, in feet, at any point of observation in the vicinity of a well discharging at a constant rate

Q = discharge of a well, Imperial gallons per minute

T = transmissibility, in Imperial gallons.day. - Foot-

r = distance, in feet, from the discharging well to the point of observation

5 = coefficient of storage

t = time, in days, since discharge started

The non-equilibrium formula is based upon the following assumptions:

- (1) the aquifer is homogeneous and isotopic
- (2) the aquifer has infinite areal extent
- (3) the discharge or recharge well penetrates and receives or transmits water from the entire thickness of the equifor
- (4) the coefficient of transmissibility is constant at all times and at all places

- (5) the well has an infinitesimal diameter
- (6) water removed from storage is discharged instantaneously with the decline in head.

Equation 2 cannot be integrated directly but its value may be given by the series

$$\int_{0}^{\frac{1}{2}-u} du = W(u) = -0.577216 - \ln u + u - \frac{u^{2}}{2.21} + \frac{u^{3}}{3.31} \dots (3)$$

W (u) = well function of u

$$u = 1.56 r^2 5$$

The expression may be solved graphically by superposition of the observed data consisting of log s plotted against  $\frac{r^2}{t}$ , on a type curve constructed by plotting values for  $\forall (u)$  against u. The observed data are plotted, and the curve fitted to the type curve; the values for the four unknowns may then be directly obtained and inserted in the following equations:

$$s = \frac{114.6 \, Q}{I} \quad \forall (u) \tag{4}$$

$$\frac{r^2}{t} = \frac{7}{1.365} \quad u \tag{5}$$

to obtain the numerical values for the transmissibility and storage coefficient. If the radius to the point of observation is small (for example, in a pumping well) or if the time elapsed since pumping started is large ( $u \in 0.02$ ), then Theiss equation may be simplified to

$$T = \frac{254 \, Q}{\Lambda^{5}}$$

T = transmissibility, imperial gallons.day. foot

Q = pumping rate, Imperial gallons per minute

 $\Delta$  s = change in drawdown in feet, in one log cycle of time.

The data are plotted on semi-logarithmic graph paper, the observed drawdown on

the arithmetic scale against the time in minutes since pumping started on the logarithmic scale. The points should plot on a straight line.

The maximum safe continuous pumping rate for a <u>single</u> well in an artesian aquifer should not exceed

$$Q_{max} = \frac{Ts_1}{2110} gpm$$

Q max = maximum safe continuous production rate, gpm.

T = transmissibility, gallons.day .foot .

to the top of the equifer.

APPENDIX C

CHEMICAL ANALYSES



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C. Segreen Schle. Prov. Analyst.



# WATER ANALYSIS REPORT CHEMICAL

Submitted l	lown Manager		Date received	Aug. 27/57	
Address	Stattler, Ata			Sept 5. 6/57:	
				A STATE COMPANY	
Container			ing and a second	AND THE RESIDENCE OF THE PROPERTY OF THE PROPE	
				57-5788	
		PARTS PER			
Total Sold		851			
Ignition Lo					
		20			
Hardness					
Sulphates		112**			
Chlorides		3			
- Alkalinity					X.
Nature of A	libalinity	Bicarbona		me and magnesiv	
Mitrites		Trace			
Nitrates		0.4			
Iron		0.1			
REMARKS	This water is	chemically sui	table. The wo	da content	
	plants and cor	er gallon) is rode aluminum.	erity argument		
			C. Emerso	n Moble,	
	-bb		Prov. Ana	lyst.	
7.	COST.	10 (10 m) 10 m) 10 m)			



WATER ANALYSIS REPORT

The state of the s

Total Ball

Suirings V

615

Bigarbonate of sode lime and magnasium Taca

Vera de

Lon This voter is chemically suitable. The soca contents (LLS) grains per gallon ) is fairly high it will have plants and corrode aluminums.

Ca Emerson Mobile, Prova Analysta



# EDMONTON, ALBERTA

AVATERSANALYSIS FEROES

Submitted by	Tom Lacros		.,,	Arte 19 State
THE PARTY OF THE P	State on Alfa			នៃជា ស៊ីស៊ី : : : : :
				J. tot
	10.0	· San G. R. Jan	control -	<u> </u>
			MATE S.	57:5790 - S
			FR(0)(2 14	
Total Sollie		987		
				74.
Hardness		20		
Sulphates		152		
- Chlorides		6		
Alkalinity		660		e and magnesium
Nature of Alkal	inity	Trace		
Nitrites		Trace		
Nitrates		NIL		
PEMARKS	This vator is c	namically suitab	le. The soda	content
1	(47.5 grains pe	r gallon) is fai corrode aluminu	rly high. It	will
72.00		Roughter Ballacter in		

C. Emerson Noble, Prov. Analyst.

-pp

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### EMERSON NOBLE

CHENICAL ENGINEERS



EDMONTON, ALBERTA

WATER ANALYSIS REPORT

CHEMICAL

Submitted 5	<b>"</b> [2] / 10 / 10 / 10 / 10 / 10 / 10 / 10 / 1		i ai ecavio		
Address			AND THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO I	4:4:22	<b>工工技工技术的工作</b>
			June of Sun		
Container 176-			Sepalation		
		<b>这是这是</b>	12 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
		PANCESSES.	IAA(a)(E		
Total Solias		- 102		+ 6.2	
ignition Loss			•		- 7.1
Hardness					
Sulphates ::	CONTRACTOR OF THE PARTY OF THE				
Chlorides ( )		7-6			
Alkalinity		580 4			
Nature of Alka	linity	Sycarophace	grisoca: L	re and mag	resium

remarks: This waver-description catery sprivaters fine social convent

REMARKS: This water is chemically suitable. The socia content, (42.3 grains per gallon is fairly high. It will scorrode aluminum and harm plants.

bb

C. -merson moble, Prov. Analyst.

-bb



EDMONTON, ALBERTA

# WATER ANALYSIS REPORT

### CHEMICAL

Submitted by	Town: Manager		Date received	Aug. 27/	7
The second second	Spetivler in Alta		West Tale		K. Transfer
			Source of San	ople F7 well	
Container No.	B&C		_echin	1832 x	
			Lab. No.		
		PARTS PER	MILLION		
Total Solids		866			
Ignition Loss		30			
Hardness		15			
Sulphates Chlorides		3			
Alkalinity		505			
Nature of Alka	linity		ate of soda.	lime and magn	esium
Nitrites -		Trace			
Nitrates		Trace:			
Iron		7.1			
REMARKS:	This water is	chemically sui	table. The s	oda content	
	(36.3 grains poaluminum.	er gallon) wil	ı narm plants	and corrode	

C. -merson Noble, Prov. Analyst.

-bb

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EDMONTON, ALBERTA

# WATER ANALYSIS REPORT

	a m	12000				Company of the Samuel	The state of the
Submitter	d by	wir manag	er,		Date received	Aug. 27/5	7 30 5 10 15
	S+						
Address	T. T.	er of er	ilta.		Date reported	Sept. 6/5	
	11.		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			a watt	4.4
And the same of	HANNIN -	3-300			Source of Sampl	CALL THE STATE OF	
	No v	BVC .			<u></u>	71835 X	
Container	***			4. 14. 14. 14. 14. 14. 14. 14. 14. 14. 1		C 4 (0.4 (p)	The state of
					ab No.	57-5796	
			PAR	S PER MILLI	ON		
		100					The second
Total Soli	ds		7.5 1 934				
		S. Carre	$\mathbf{x}_{i,j} = \mathbf{x}_{i,j}$				
Ignition L	OSS		37.3				
Hardness		7 16 A 16	1103				
TIMI (III SS)							
Sulphates	VIV		119-				
V.							
Chlorides		Secretary and the secretary of the secre	6				
en e	THE PARTY OF		W. P. C.				
Alkalinity			595				432
					100		
Nature of	Alkalinity		Bica	rbonate of	soda, lime	and magne	esium 🖖 📜
. 4	2		A STEWNITE				
Nitrites '			NIL				
			e es enils				
Nitrates							
Iron			1.0.1				
1.011 · 3							
REMIARK	S. This	water fe	chemically	suitable	The sode	content 1	
			er gallon a				
,	plant	S					
•							

C. Emerson Noble, Prov. Analyst.

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C. EMERSON NOBLE
CHEMICAL ENGINEERS
DIRECTOR INDUSTRIAL LABORATORIES
PROVINCIAL AMALYST

COPY



EDMONTON, ALBERTA

# WATER ANALYSIS REPORT

1	Submitted by	Town Manag	er,	Aug. 27/57	
	Market Company of the	Stettler	我们一个企业的现在分词 在中国人的企业工作的 一至15 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Date reported Sept. 6/57	
27				Source of Sample	
A	Container No.	B&C		Serial No. 1834 x	
				Lab No. 57-5797	Y
			PARTS PER MIL	LION	
4. 11.	Total Solids		898		
	Ignition Less		44		
	Hardness		202		
إ	Sulphates Chlorides		2		
	Alkalinity		475		
	Nature of Alka	linity	Bicarbonate	of soda, lime and magnesi	um
	Nitrites VI		NIL		
, y .	Nitrates		NIL		
	Irca		0.1		
	REMARKS:	This water 33.7 grains	is chemically suitable per gallon and will	le. The soda content is corrode aluminum and harm	
		plants.			1000 A 1
ř					
,		-bb		C. Emerson Noble, Prov. Analyst.	
		*			18 1



# EDMONTON, ALBERTA

# WATER ANALYSIS REPORT

ubmitted		own Man	ager, 🛸		Da	te received	Aug	27/57	1
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				第23章					
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					5 2 6 6		a Ly		
otal Soli	d.			878					
وكما المدائدة الديا	The second second								
mition L	088								
ardness				10	-3i		4		
					Agent age	100			
ulphates				137					7
hiorides				8					
lkalinity	and and the second seco			555	47	78.00			- <del> </del>
atura of	Alkalinity	1.470		Bicarbo	nate of	soda1	ime and	magnes:	ium
ature or					Total Market				
itrites			and the second s	NIL		State of the second of the sec			
el				NIL					
Harates Harates									
on									
ET EA ER	C. m	ACTION OF STREET OF							
enataren	S. Thee	s water	rs cnem	The soda	i content	iter the	e iron n 5 grains	145	We
	per	gallon	and will	l corrode	aluminu	um and ha	arm plan	its.	
1). 2)								7. 4	100 mg
	ontainer ont	ontainer Nos  ontainer Nos  otal Solids  mition Loss  ardness  allohates  hlorides  lkalinity  ature of Alkalinity  itrites  itrates  on  EMARKS: Thi	ontainer No  Baccontainer No  cal Solids  mition Loss  ardness  allohates  hlorides  lkalinity  ature of Alkalinity  itrites  itrates  on  EMARKS: This water  been settle	ontainer No.  Decontainer No.  Decontain	Statises, Altacky  ontainer No.  PARTS Experiments and the second and second	ddress Status Altary Da  So  So  Data Solids PARTS EER MILLION  PARTS EER MILLION  pardness 10  ardness 10  liphates 137  allorides 8  likalinity 555  ature of Alkalinity Bicarbonate of NIL  itrates NIL  EMARKS: This water is chemically suitable a been settled out. The soda content	ontainer No.  BC  Serial No.  Lab. No.  PARTS EER MILLION  Tables  Inition Loss  Inliners  Inlin	ddress Stattler, Altas Date reported Sept.  Source of Sample #11  Container Mo BCC Serial No.  PARTS EXR MILLION 77  PARTS EXR MILLION 78  PARTS EXR MILLI	BEC Serial No. 1835/x Serial N

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C. Emerson Noble, Prov. Analyst.

# C EMERSON NOBLE



# EDMONTON, ALBERTA

hovember 2, 1959

# WATER ANALYSIS REPORT CHEMICAL

			w. A. Mene	ley	Dat	e received	October 28,	1959
<b>2.</b> 4			nesearch. U	ouncil of Alb		e reported		
	Address			& 87 Avenue,		The same of the same of the same	59 2	NOLE (35)
	Container				Ser	ial No.		
	Container				Lat	No. 5	9 - 8255.	
				PARTS	PER MILLION			
1. 4 A. A.	Total Solid	ds -	706					
	Ignition L	46.7	A STATE OF THE STA					
10 mg	Hardness Sulphates		10 28					
ĺ	Chlorides	***	Nil					
	Alkalinity	at 4	575					
Į.	Nature of	Alkaliı			onate of so	oda, lime a	ind magnesium	
	Nitrites		, "i]					
1	Nitrates		. Ail					
FFT (A.)	Iron							
	Fluorine REMARK	rs:						
-		71.	provide a		100	0.2	as 5	
	terning repri		water chem	ically suitab		h will ham	m plants and	corrode
	5		soda = 4	ically suitar	gallon which	n will har	m plants and	corrode

Provincial Analyst.

# Instructions for Riley's

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